

Analytical investigations of plane wave propagation across a single tuned mass damper

BERNARDINO CHIAIA

Politecnico di Torino
Dip. di Ingegneria Strutturale,
Edile e Geotecnica
Corso Duca degli Abruzzi, 24
10129 Torino, ITALY
bernardino.chiaia@polito.it

LUCA PLACIDI

International Telematic
University Uninettuno
Engineering Faculty
C.so Vittorio Emanuele II, 39
00186 Roma, ITALY
luca.placidi@uninettunouniversity.net

LIVIO CONTI

International Telematic
University Uninettuno
Engineering Faculty
C.so Vittorio Emanuele II, 39
00186 Roma, ITALY
livio.conti@uninettunouniversity.net

Abstract: The problem of wave propagation in elastic beams is presented in a general fashion. The role of tuned mass damper is also shown. In particular we will calculate reflection and transmission coefficients across a tuned mass damper and show their frequency dependence. The same analysis will be extended to the case of an impact tuned mass damper. In particular we will show the effect of impact on the equations that govern this kind of devices.

Key-Words: Second gradient, Elasticity, Variational approach, Wave, dispersion relation

1 Introduction

In [40, 55] it is proposed a general set of equations, obtained via a variational principle, accounting for surface mass density, elasticity and inertia embedded in a three-dimensional second gradient material [2, 37, 38, 42, 59]. In particular, well posed duality jump conditions to be imposed at the considered structured surfaces are introduced and discussed. In this contribution we apply the same strategy to the case of Euler beams. Euler beams are treated as second gradient materials, i.e. as materials for which the internal energy depends upon the second gradient of the placement field [3, 24, 25, 46, 52, 53, 54, 60]. In Euler beams the body is one-dimensional. Thus, the internal energy of an Euler beam is assumed to depend simply on the second derivative of the (dis)placement field. Classic variational approach has been used to derive the system of PDEs and boundary conditions. Many types of internal constraints across two semi-

infinite Euler beams can be considered. In this paper we consider the case of a Tuned Mass Damper (TMD), that is a very efficient technique to reduce wave propagation [1, 56]. Tuned mass dampers are devices mounted in structures to reduce the amplitude of mechanical vibrations. They are frequently used in many fields of engineering. Various vibration control techniques may be used in order to reduce wave propagation in beams [49]. Mass-spring systems are widely used to control the response of resonant structures [48, 49, 50]. According to Ormoundroyd and Den Hartog [51], the use of TMDs was first suggested in 1909.

At its resonance frequency an undamped mass-spring system is attached to the host structure and it should be tuned to its resonance. However, to give the best effect over a frequency band under random excitation, Den Hartog [45] derived optimum values for the frequency of a damped absorber and its damping ratio in order to minimize the displacement

response of the host structure. This paper gives an easier method to achieve the same goal.

2 Wave propagation in 1D second gradient elasticity (Euler beam)

2.1 Formulation of the problem

X is the coordinate of the material points of the 1D body in the reference configuration, L is its length and $X \in [0, L]$. t is the time and $t \in [t_i, t_f]$, where t_i and t_f are initial and final time of the dynamic process, respectively. The kinetic energy density functional $K(\dot{u})$ depends only on the first derivative \dot{u} , i.e. on the velocity, of the displacement field $u(X, t)$ with respect to t . The internal energy density functional $U(u'')$ depends only on the second derivative u'' of the displacement field $u(X, t)$ with respect to X . The action functional $\mathcal{A}(u(X, t))$ is given by the contributions of kinetic, internal and the external energies as follows,

$$\begin{aligned} \mathcal{A}(u(X, t)) = & \int_{t_i}^{t_f} \left\{ \int_0^L [K(\dot{u}) - U(u'')] \right. \\ & + b^{ext}u + m^{ext}u' \Big] dX \\ & + F_0^{ext}u(0, t) + F_L^{ext}u(L, t) \\ & \left. + M_0^{ext}u'(0, t) + M_L^{ext}u'(L, t) \right\} dt \end{aligned} \quad (1)$$

where $b^{ext}(X)$ and $m^{ext}(X)$ are the external distributed force and couple, F_0^{ext} , F_L^{ext} are the external concentrated forces at $X = 0$ and at $X = L$, respectively, and M_0^{ext} , M_L^{ext} are the external concentrated couples at $X = 0$ and at $X = L$, respectively. If we assume $\delta\mathcal{A} = 0$ for any admissible variation δu then from (1) we get the final form of the system of partial differential equations, that can be explained once kinematical restrictions are defined. Kinetic energy density K is assumed to be quadratic in the velocity \dot{u} ,

$$K(\dot{u}) = \frac{1}{2} \rho \dot{u}^2, \quad (2)$$

where the coefficient $\rho(X)$ is the so-called mass density of the material. Internal energy density is assumed to be quadratic in the so-called strain gradient u'' ,

$$U(u'') = \frac{1}{2} K_M u''^2,$$

where the coefficient $K_M(X)$ is the so-called bending stiffness of the material.

If the displacement field $u(X, t)$ is interpreted as transverse to the direction of the line defined by the reference configuration of the material body, and if such a body is composed by isotropic elastic material with Young modulus E and I is the moment of inertia of its cross section, then we have $K_M = EI$. Finally, we will consider only the case of admissible variation δu such that

$$\delta u(X, t_i) = \delta u(X, t_f) = 0 \quad (3)$$

The result is given by reporting the variation of the action functional,

$$\begin{aligned} \delta\mathcal{A}(u(X, t)) = & - \int_{t_i}^{t_f} \left\{ \int_0^L [\rho \ddot{u} + (K_M u^{II})^{II}] \right. \\ & - b^{ext} + (m^{ext})^I \Big] \delta u dX \\ & + [(K_M u^{II} - M_L^{ext}) \delta u']_{X=L} \\ & - [(K_M u^{II} + M_0^{ext}) \delta u']_{X=0} \\ & - \left[[(K_M u^{II})^I + F_L^{ext} + m^{ext}] \delta u \right]_{X=L} \\ & \left. + \left[[(K_M u^{II})^I - F_0^{ext} + m^{ext}] \delta u \right]_{X=0} \right\} dt. \end{aligned} \quad (4)$$

2.2 Dispersion relation of the Euler beam problem

Let us assume no external actions,

$$b^{ext} = m^{ext} = 0,$$

and an indefinite length, i.e. no boundary conditions are considered. Thus, the Partial Differential Equations PDEs are,

$$\rho \ddot{u} + (K_M u^{II})^{II} = 0, \quad \forall X, t. \quad (5)$$

Let us, now, look for plane wave solution, for the homogeneous ($K'_M = 0$) case, in the following form,

$$u(X, t) = \text{Re} \{ u_0 \exp [I(\omega t - kX)] \}, \quad (6)$$

where I is the imaginary unit, ω the frequency, k the wave number, and insert (6) into (5),

$$\text{Re} \{ (-\rho \omega^2 + K_M k^4) u_0 \exp [I(\omega t - kX)] \} = 0. \quad (7)$$

Thus, the (7) is given in a more suitable way,

$$(-\rho\omega^2 + K_M k^4) u = 0. \quad (8)$$

The (8) is satisfied for every displacement field if and only if,

$$k^4 = \frac{\rho}{K_M} \omega^2, \quad (9)$$

that is the wanted dispersion relation.

If ω is real, then the 4 possible wave numbers (solutions of the dispersion relation (9)) are,

$$k = k_{1,2,3,4}, \quad k_{1,2} = \pm \bar{k},$$

$$k_{3,4} = \pm I\bar{k}, \quad \bar{k} = \sqrt[4]{\frac{\rho}{K_M} \omega^2} \in R^+$$

that correspond to four possible waves. A general solution in terms of plane waves is therefore,

$$\begin{aligned} u(X, t) = & \text{Re}\{u_{01} \exp [I(\omega t - k_1 X)] \\ & + u_{02} \exp [I(\omega t - k_2 X)] \\ & + u_{03} \exp [I(\omega t - k_3 X)] \\ & + u_{04} \exp [I(\omega t - k_4 X)]\} \end{aligned}$$

that is also

$$\begin{aligned} u(X, t) = & \text{Re}\{u_{01} \exp [I(\omega t - \bar{k} X)] \\ & + u_{02} \exp [I(\omega t + \bar{k} X)] \\ & + u_{03} \exp [I(\omega t - I\bar{k} X)] \\ & + u_{04} \exp [I(\omega t + I\bar{k} X)]\} \end{aligned}$$

or

$$\begin{aligned} u(X, t) = & \text{Re}\{\exp [I\omega t] [u_{01} \exp (-I\bar{k} X) \\ & + u_{02} \exp (I\bar{k} X) + u_{03} \exp (\bar{k} X) \\ & + u_{04} \exp (-\bar{k} X)]\} \end{aligned} \quad (10)$$

It is easy to show that the amplitude u_{01} corresponds to a plane wave propagating towards the positive axis X and, vice versa, the amplitude u_{02} corresponds to a plane wave propagating towards the negative axis X . Besides, the amplitudes u_{03} and u_{04} correspond to the so-called standing waves, being the standing wave associated to the amplitude u_{03} diverges to infinity at $X \rightarrow +\infty$ and that to the amplitude u_{04} diverges at $X \rightarrow -\infty$.

2.3 Energy and average energy fluxes related to plane waves

The calculation of the energy fluxes is done as follows. First of all, we define the total energy density, that is the sum of kinetic and internal energy. The time derivative of the total energy density is

$$\dot{E} = \rho \dot{u} \ddot{u} + K_M u'' \dot{u}'', \quad (11)$$

and the flux H is defined in such a way that the following PDE is satisfied,

$$\dot{E} + H' = 0 \quad (12)$$

By the use of the PDE (5) of the process we have

$$\dot{E} = -\dot{u} (K_M u''')^I + K_M u'' \dot{u}''. \quad (13)$$

By the use of the chain derivative rule we have,

$$\begin{aligned} \dot{E} = & -\left[\dot{u} (K_M u''')^I\right]^I + \dot{u}' (K_M u'')^I \\ & + (K_M u'' \dot{u}')^I - (K_M u'')^I \dot{u}' \\ = & -\left[\dot{u} (K_M u''')^I\right]^I + (\dot{u}' K_M u'')^I \\ = & \left(-\dot{u} (K_M u'')^I + \dot{u}' K_M u''\right)^I \end{aligned} \quad (14)$$

By comparison of (12) and (14), we have

$$H = \dot{u} (K_M u'')^I - \dot{u}' K_M u''. \quad (15)$$

The average flux density $\langle H \rangle$ related to a wave with frequency ω is defined by the integration of H in time over the period $T = 2\pi/\omega$ of the wave,

$$\langle H \rangle = \int_t^{t+T} H(X, \tilde{t}) d\tilde{t}. \quad (16)$$

2.3.1 Energy flux related to a propagative wave towards the positive direction of the axis X

Let us take into account the propagative wave towards the positive direction of the axis X , with $\omega, \bar{k} \in Real^+$,

$$u(X, t) = \text{Re} (u_0 \exp (I(\omega t - \bar{k} X))). \quad (17)$$

Thus, its average density flux is defined by (15) and (16),

$$\langle H \rangle = \left\langle \dot{u} (K_M u'')^I \right\rangle - \left\langle \dot{u}' K_M u'' \right\rangle,$$

that for the homogenous case is,

$$\langle H \rangle = K_M (\langle \dot{u} u^{III} \rangle - \langle \dot{u}' u'' \rangle). \quad (18)$$

By insertion of (17) into (18) and by using the theorem (33) defined in the Appendix2 in Sect. 4,

$$\frac{\langle H \rangle}{K_M \omega \bar{k}^3} = \frac{1}{2} \text{Re}(u_0 u_0^*) + \frac{1}{2} \text{Re}(u_0 u_0^*) = \|u_0\|^2,$$

that is

$$\langle H \rangle = K_M \omega \bar{k}^3 \|u_0\|^2,$$

where $\|u_0\|$ is the modulus of the amplitude u_0 and u_0^* is its complex conjugated.

Thus, we remark that the average energy flux of a propagative wave towards the positive direction of the axis X is positive.

2.3.2 Energy flux related to a propagative wave towards the negative direction of the axis X

Let us take into account the propagative wave towards the negative direction of the axis X , with $\omega, \bar{k} \in \text{Real}^+$

$$u(X, t) = \text{Re}(u_0 \exp(I(\omega t + \bar{k}X))). \quad (19)$$

Its energy flux (18) is now computed. By insertion of (19) into (18) and by using the theorem (33) defined in the Appendix2 of Sect. 4,

$$\frac{\langle H \rangle}{K_M \omega \bar{k}^3} = -\frac{1}{2} \text{Re}(u_0 u_0^*) - \frac{1}{2} \text{Re}(u_0 u_0^*) = -\|u_0\|^2,$$

that is

$$\langle H \rangle = -K_M \omega \bar{k}^3 \|u_0\|^2,$$

where $\|u_0\|$ is the modulus of the amplitude u_0 , and u_0^* is its complex conjugated.

Thus, we remark that the average energy flux of a propagative wave towards the negative direction of the axis X is negative.

2.3.3 Energy flux related to a general standing wave

Let us take into account a general standing wave, with $\omega, \bar{k} \in \text{Real}^+$

$$u(X, t) = \text{Re}(u_0 \exp(I\omega t \pm \bar{k}X)). \quad (20)$$

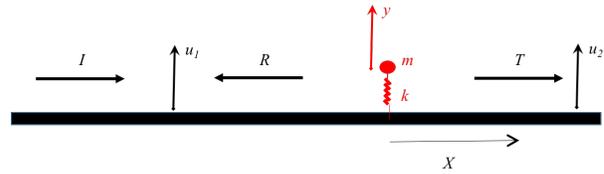


Figure 1: A mass-spring system with rigidity k and mass m inside an indefinite beam at the position $X = 0$. Incident, reflection and transmission coefficients are made explicit.

Its energy flux (18) is now computed. By insertion of (20) into (18), and by using the theorem (33) defined in the Appendix, we have

$$\langle H \rangle = 0,$$

where $\|u_0\|$ is the modulus of the amplitude u_0 , and u_0^* is its complex conjugated.

Thus, we remark that the average energy flux of a general standing wave is null.

2.4 Reflection and transmission coefficients

Reflection R and transmission T coefficients are related to the average energy fluxes of the reflected and transmitted waves, respectively. Thus they do not consider the standing waves solution, because their average density flux is null. If we assume that H_i is the energy flux of an incident wave and H_r and H_t are the energy fluxes of the reflected and transmitted waves, the following definition hold,

$$R = \left\| \frac{\langle H_r \rangle}{\langle H_i \rangle} \right\|, \quad T = \left\| \frac{\langle H_t \rangle}{\langle H_i \rangle} \right\|. \quad (21)$$

3 The case of an indefinite beam with a tuned mass damper

3.1 Formulation of the problem

Let us consider a mass-spring system with rigidity k and mass m inside an indefinite beam at the position $X = 0$, see the Fig. 1. The displacement field must now be treated with two independent functions, $u_1(X, t)$ and $u_2(X, t)$. The first related to the displacement field on the left-hand side

and the second to that of the right-hand side. The action $\mathcal{A}(u_1(X, t), u_2(X, t))$ is the superposition of $(\mathcal{A}_1(u_1(X, t)),$

$$\mathcal{A}_1(u_1(X, t)) = \int_{t_i}^{t_f} \left\{ \int_{-L}^0 \left[\frac{1}{2} \rho \dot{u}_1^2 - \frac{1}{2} K_M u_1''^2 \right] dX \right\}$$

and $\mathcal{A}_2(u_2(X, t)),$

$$\mathcal{A}_2(u_2(X, t)) = \int_{t_i}^{t_f} \left\{ \int_0^L \left[\frac{1}{2} \rho \dot{u}_2^2 - \frac{1}{2} K_M u_2''^2 \right] \right\}$$

related to both sides of the indefinite beam and to that of the internal constraint spring mass system $\mathcal{A}_{IC},$

$$\mathcal{A} = \mathcal{A}_1 + \mathcal{A}_2 + \mathcal{A}_{IC} \quad (22)$$

The vertical displacement of the mass m is not constrained to be attached to the beam but it is given by its vertical coordinate $y(t),$ that is a function of time. The internal clamp constrained is considered

$$u_1(0, t) = u_2(0, t), \quad u_1'(0, t) = u_2'(0, t), \quad (23)$$

between the two sides of the indefinite beam. and the action of the mass spring system is therefore,

$$\mathcal{A}_{IC}(\cdot) = \frac{1}{2} m [\dot{y}(t)]^2 - \frac{1}{2} k (y(t) - u_1(0, t))^2 \quad (24)$$

where no distributed nor concentrated external actions are considered, the length L will be assumed to be arbitrary large, i.e. $L \rightarrow \infty$ and the mass-spring system is concentrated at $X = 0$ where the displacement $u_1(0, t) = u_2(0, t)$ and the velocity $\dot{u}_1(0, t) = \dot{u}_2(0, t)$ are the same in the two independent branches of the beam. The variation of (24),

$$\delta \mathcal{A}_{IC}(\cdot) = \delta y [-m\ddot{y} - ky + ku_1] + \delta u_1(0, t) k [y(t) - u_1(0, t)]$$

is used to derive the variation of (22),

$$\begin{aligned} \delta \mathcal{A}(u_1(X, t), u_2(X, t), y(t)) = & \\ - \int_{t_i}^{t_f} \left\{ \int_0^L \left\{ \delta u_1 \left[\rho \ddot{u}_1 + (K_M u_1^{II})^{II} \right] \right. \right. & \\ \delta u_2 \left[\rho \ddot{u}_2 + (K_M u_2^{II})^{II} \right] \Big\} dX & \\ + \left[(K_M u_1^{II}) \delta u_1' - (K_M u_2^{II}) \delta u_2' \right]_{X=0} & \\ - \left[\left[(K_M u_1^{II})^I \right] \delta u_1 - \left[(K_M u_2^{II})^I \right] \delta u_2 \right]_{X=0} & \\ \delta y [-m\ddot{y} - ky + ku_1] & \\ + \delta u_1(0, t) k [y(t) - u_1(0, t)] \Big\} dt. & \end{aligned}$$

Thus, the PDEs are

$$\rho \ddot{u}_1 + (K_M u_1^{II})^{II} = \rho \ddot{u}_2 + (K_M u_2^{II})^{II} = 0$$

and the boundary conditions at $X = 0,$ because of the kinematical restrictions in (23) are

$$\begin{aligned} u_1(0, t) = u_2(0, t), \quad u_1'(0, t) = u_2'(0, t), \\ u_1^{II}(0, t) = u_2^{II}(0, t), \\ -m\ddot{y}(t) - ky(t) + ku_1(0, t) = 0 \end{aligned} \quad (25)$$

$$\begin{aligned} -K_M u_1^{III}(0, t) + K_M u_2^{III}(0, t) \\ + k [y(t) - u_1(0, t)] = 0 \end{aligned} \quad (26)$$

3.2 A plane wave solution

Let us assume a propagative wave towards the right-hand side (i.e. towards the positive direction of the X axis) into the left-hand side of the beam. In this way, such a propagative wave complies the mass-spring system and interact with it. Such an incident wave has, therefore, the form of (17), i.e., (with $\omega, \bar{k} \in Real^+$)

$$u_i(X, t) = \text{Re} (u_i^p \exp (I (\omega t - \bar{k} X))) . \quad (27)$$

Once the incident wave complies the concentrated spring-mass system, a transmitted and a reflected wave is generated. However, not all the 4 kind of waves in (10) can be generated on both sides of the concentrated mass. First of all, the standing wave that diverges at $X \rightarrow -\infty$ is not possible (Sommerfield condition) for the reflected wave and the standing wave that diverges at $X \rightarrow +\infty$ is not possible (Sommerfield condition) for the transmitted wave. Thus the reflected wave can only be of the following form,

$$\begin{aligned} u_r(X, t) = \text{Re}(u_r^p \exp (I (\omega t + \bar{k} X))) \\ + u_r^s \exp (I \omega t + \bar{k} X), \quad \omega, \bar{k} \in Real^+. \end{aligned}$$

and the transmitted wave is given by,

$$\begin{aligned} u_t(X, t) = \text{Re}(u_t^p \exp (I (\omega t - \bar{k} X))) + \\ + u_t^s \exp (I \omega t - \bar{k} X), \quad \omega, \bar{k} \in Real^+. \end{aligned}$$

Thus, in the vicinity of the mass resonator, the solution is the superposition $u_i(X, t) + u_r(X, t)$ of

incident $u_i(X, t)$ and reflected $u_r(X, t)$ waves for the left-hand side of the concentrated mass,

$$u_1(X, t) = u_i(X, t) + u_r(X, t) = \\ \operatorname{Re}(u_i^p \exp(I(\omega t - \bar{k}X)) + u_r^p \exp(I(\omega t + \bar{k}X)) \\ + u_r^s \exp(I\omega t + \bar{k}X))$$

and simply the transmitted wave for the right-hand side of the concentrated mass, (with $\omega, \bar{k} \in \text{Real}^+$)

$$u_2(X, t) = u_t(X, t) = \\ \operatorname{Re}(u_t^p \exp(I(\omega t - \bar{k}X)) + u_t^s \exp(I\omega t - \bar{k}X)).$$

The oscillation of the internal resonator is also achieved,

$$y(t) = y_0 \exp(I\omega t) \quad (28)$$

3.3 Reflection and transmission condition across a tuned mass damper

The amplitude of the propagative incident wave u_i^p is again considered a datum of the problem. In the following we will find both the amplitudes of reflected u_r^p and u_r^s and of the transmitted u_t^p and u_t^s waves, as well as the amplitude y_0 of the internal resonator. The boundary conditions are the 5 included in eqns. (25), (25) and (26). By insertion of (28), (28) and (28) into eqns. (25), (25) and (26) we derive the five equations that are needed to derive the unknowns of reflected u_r^p and u_r^s , of the transmitted u_t^p and u_t^s and of the internal

resonator y_0 waves. Keeping in mind that

$$u_1' = \operatorname{Re}(u_i^p (-I\bar{k}) \exp(I(\omega t - \bar{k}X)) \\ + u_r^p (I\bar{k}) \exp(I(\omega t + \bar{k}X)) \\ + u_r^s (\bar{k}) \exp(I\omega t + \bar{k}X)) \\ u_1'' = \operatorname{Re}(u_i^p (-I\bar{k})^2 \exp(I(\omega t - \bar{k}X)) \\ + u_r^p (I\bar{k})^2 \exp(I(\omega t + \bar{k}X)) \\ + u_r^s \bar{k}^2 \exp(I\omega t + \bar{k}X)) \\ u_1''' = \operatorname{Re}(u_i^p (-I\bar{k})^3 \exp(I(\omega t - \bar{k}X)) \\ + u_r^p (I\bar{k})^3 \exp(I(\omega t + \bar{k}X)) \\ + u_r^s \bar{k}^3 \exp(I\omega t + \bar{k}X)) \\ \dot{u}_1 = \operatorname{Re}(u_i^p (I\omega) \exp(I(\omega t - \bar{k}X)) \\ + u_r^p (I\omega) \exp(I(\omega t + \bar{k}X)) \\ + u_r^s (I\omega) \exp(I\omega t + \bar{k}X)) \\ \ddot{u}_1 = \operatorname{Re}(u_i^p (I\omega)^2 \exp(I(\omega t - \bar{k}X)) \\ + u_r^p (I\omega)^2 \exp(I(\omega t + \bar{k}X)) \\ + u_r^s (I\omega)^2 \exp(I\omega t + \bar{k}X))$$

and

$$u_2' = \operatorname{Re}(u_t^p (-I\bar{k}) \exp(I(\omega t - \bar{k}X)) \\ + u_t^s (-\bar{k}) \exp(I\omega t - \bar{k}X)) \\ u_2'' = \operatorname{Re}(u_t^p (-I\bar{k})^2 \exp(I(\omega t - \bar{k}X)) \\ + u_t^s (-\bar{k})^2 \exp(I\omega t - \bar{k}X)) \\ u_2''' = \operatorname{Re}(u_t^p (-I\bar{k})^3 \exp(I(\omega t - \bar{k}X)) \\ + u_t^s (-\bar{k})^3 \exp(I\omega t - \bar{k}X)) \\ \dot{u}_2 = \operatorname{Re}(u_t^p (I\omega) \exp(I(\omega t - \bar{k}X)) \\ + u_t^s (I\omega) \exp(I\omega t - \bar{k}X)) \\ \ddot{u}_2 = \operatorname{Re}(u_t^p (I\omega)^2 \exp(I(\omega t - \bar{k}X)) \\ + u_t^s (I\omega)^2 \exp(I\omega t - \bar{k}X))$$

and, without loss of generality, assuming the time $t = 0$ and position $X = 0$, the five conditions (25), (25)

and (26) are as follows,

$$\begin{aligned}
u_i^p + u_r^p + u_r^s &= u_t^p + u_t^s, \\
u_i^p (-I\bar{k}) + u_r^p (I\bar{k}) + u_r^s (\bar{k}) \\
&= u_t^p (-I\bar{k}) + u_t^s (-\bar{k}), \\
u_i^p (-I\bar{k})^2 + u_r^p (I\bar{k})^2 + u_r^s \bar{k}^2 \\
&= u_t^p (-I\bar{k})^2 + u_t^s (-\bar{k})^2, \\
u_i^p (-I\bar{k})^3 + u_r^p (I\bar{k})^3 + u_r^s \bar{k}^3 \\
&= u_t^p (-I\bar{k})^3 + u_t^s (-\bar{k})^3 \\
&+ \frac{k}{K_M} [y_0 - u_i^p - u_r^p - u_r^s] \\
&- m y_0 (I\omega)^2 - k y_0 + k (u_i^p + u_r^p + u_r^s)
\end{aligned}$$

that is possible to solve analytically,

$$u_r^p = -\frac{(1+I)km\omega^2}{D_1} u_i^p, \quad (29)$$

$$\begin{aligned}
D_1 &= 4(-1+I)kK_M\bar{k}^3 \\
&+ 2(k+2(1-I)K_M\bar{k}^3)m\omega^2 \\
u_r^s &= -\frac{(1-I)km\omega^2}{D_2} u_i^p, \quad (30)
\end{aligned}$$

$$\begin{aligned}
D_2 &= 4(-1+I)kK_M\bar{k}^3 \\
&+ 2(k+2(1-I)K_M\bar{k}^3)m\omega^2 \\
u_t^p &= \frac{(1+I)(4kK_M\bar{k}^3 - (k+4K_M\bar{k}^3)m\omega^2)}{D_3} u_i^p,
\end{aligned}$$

$$\begin{aligned}
D_3 &= 4(1+I)kK_M\bar{k}^3 \\
&- 2I(k+2(1-I)K_M\bar{k}^3)m\omega^2
\end{aligned}$$

$$u_t^s = -\frac{(1-I)km\omega^2}{D_4} u_i^p,$$

$$\begin{aligned}
D_4 &= (-1+I)kK_M\bar{k}^3 \\
&+ 2(k+2(1-I)K_M\bar{k}^3)m\omega^2
\end{aligned}$$

$$y_0 = \frac{2(1+I)kK_M\bar{k}^3}{D_5} u_i^p$$

$$\begin{aligned}
D_5 &= 2(1+I)kK_M\bar{k}^3 \\
&- I(k+2(1-I)K_M\bar{k}^3)m\omega^2,
\end{aligned}$$

3.4 Numerical representation of reflection and transmission coefficients

By the use of the definitions (21) of the reflection and transmission coefficients, it is possible to verify the

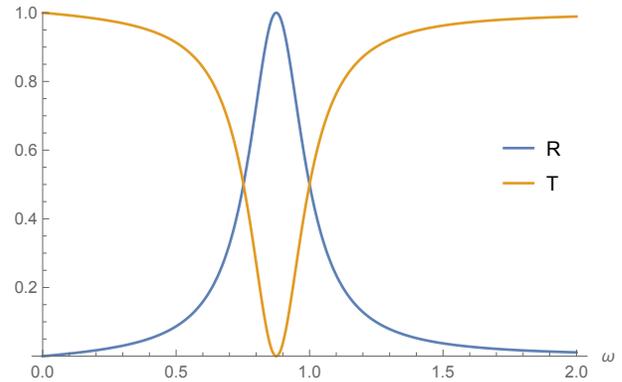


Figure 2: Reflection and Transmission coefficients with the following numerical characterization, $K_M = 1, \rho = 1, m = 1, k = 1$.

energy conservation,

$$R + T = 1.$$

Besides, reflection and transmission coefficients can be plotted in the following figures 2, 3, 4, 5 and 6.

4 Towards the design of piece-wise smooth tuned mass damper and outlook

Finally, we consider the case of the piece-wise smooth tuned mass dampers [5, 6, 7, 8, 9, 11, 12]. In particular we assume that the mass damper can only vibrate at the top of the beam. This will be modeled by a different assumption of the internal constraint action (24),

$$\begin{aligned}
\mathcal{A}_{IC} &= \frac{1}{2}m[\dot{y}(t)]^2 \\
&- \frac{1}{2}kH(y(t) - u_1(0,t))(y(t) - u_1(0,t))^2 \\
&- \frac{1}{2}k_{imp}H(u_1(0,t) - y(t))(y(t) - u_1(0,t))^2
\end{aligned} \quad (31)$$

where the Heaviside function H has been used and the rigidity $k_{imp} \gg k$ is the so called impact rigidity [4, 10, 13, 14, 15, 57].

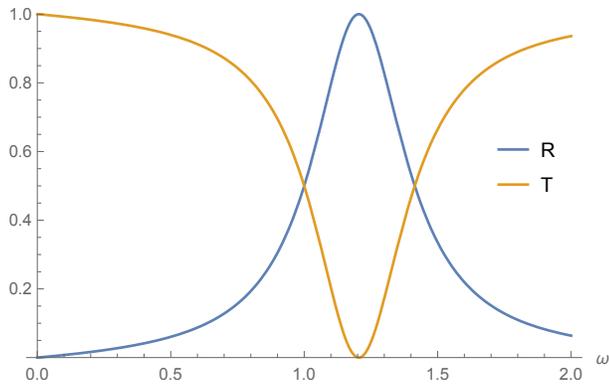


Figure 3: Reflection and Transmission coefficients with the following numerical characterization, $K_M = 1, \rho = 1, m = 1, k = 2$.

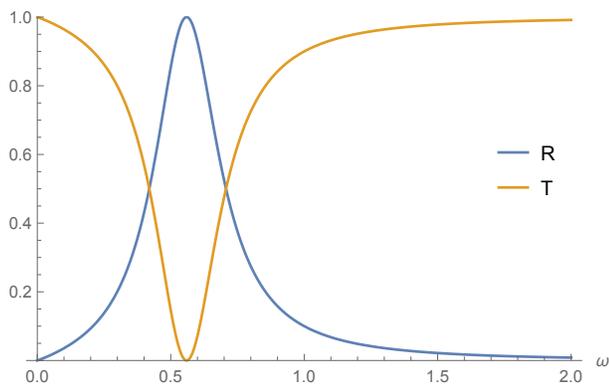


Figure 4: Reflection and Transmission coefficients with the following numerical characterization, $K_M = 1, \rho = 1, m = 2, k = 1$.

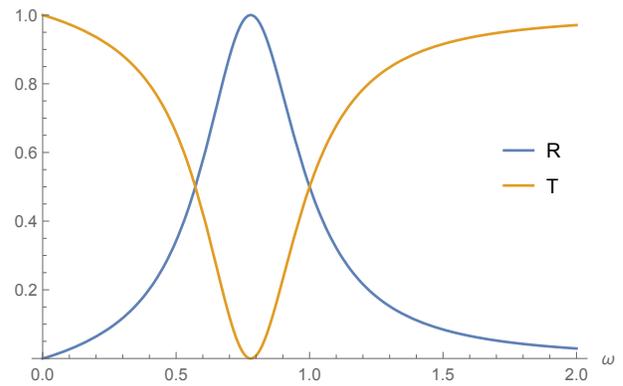


Figure 5: Reflection and Transmission coefficients with the following numerical characterization, $K_M = 0.1, \rho = 1, m = 1, k = 1$.

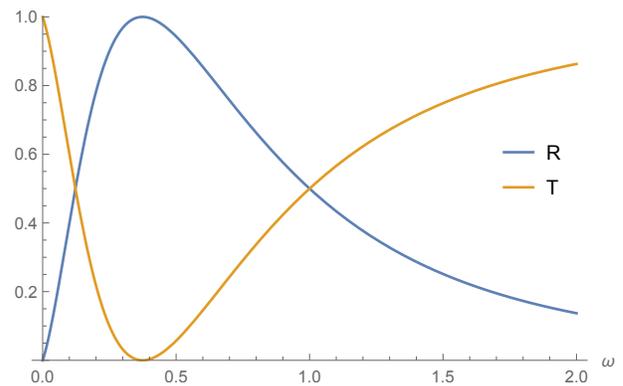


Figure 6: Reflection and Transmission coefficients with the following numerical characterization, $K_M = 1, \rho = 0.1, m = 1, k = 1$.

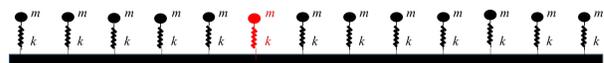


Figure 7: Distributed mass-spring system with rigidity k and mass m attached to an indefinite beam.

Distributed systems, see e.g., Fig. 7, are not new in the literature on the applications of Riccati equation [19, 22, 23] and are discussed at length in the literature on transmission lines [16, 17, 18, 20, 21]. Thus, further developments will be achieved considering a system of distributed tuned mass dampers (e.g., the oscillation of fluid into the pore space [39, 41] of different interesting metamaterials [36, 43, 44]), that could be mathematically treated with the help of special functions [26, 27, 28, 29, 30, 31, 32, 33, 34, 35].

Appendix: A theorem for the evaluation of the average flux density

Let F and f be two general complex function of this form

$$\begin{aligned} F &= F_0 \exp(I\omega t - \gamma_1), \\ f &= f_0 \exp(I\omega t - \gamma_2), \quad F_0, f_0 \in \text{Real} \end{aligned} \quad (32)$$

then it is possible to prove that

$$\int_t^{t+T} \text{Re}(F) \text{Re}(f) d\tilde{t} = \frac{1}{2} \text{Re}(F f^*), \quad (33)$$

where f^* is the complex conjugated of f .

References:

- [1] Achenbach, J.D., Wave propagation in elastic solids. In North-Hollands Series in Applied Mathematics and Mechanics, Vol. 16, Editors: Lauwerier, H.A., Koiter, W.T., North-Hollands publishing company, 1975.
- [2] J. Alibert, P. Seppecher, and F. dell'Isola, Truss modular beams with deformation energy depending on higher displacement gradients, *Mathematics and Mechanics of Solids*, vol. 8 (1), 2003, pp. 51-73.
- [3] Altenbach, H., Eremeyev, V.A., Morozov, N.F., On equations of the linear theory of shells with surface stresses taken into account, *Mechanics of Solids*, 45 (3), pp. 331-342 (2010)
- [4] U. Andreaus, P. Baragatti, L. Placidi, Experimental and numerical investigations of the responses of a cantilever beam possibly contacting a deformable and dissipative obstacle under harmonic excitation, *Int. J. of Non-Linear Mechanics* 2015 (in press).
- [5] Andreaus U., Casini P., Dynamics of friction oscillators excited by moving base or/and driving force, *J. of Sound & Vib.*, 245(4), pp. 685-699, 2001. DOI: 10.1006/jsvi.2000.3555.
- [6] Andreaus U., Casini P., Dynamics of SDOF oscillators with hysteretic motion-limiting stop, *J. of Nonlin. Dyn.*, 22, pp. 155-174, 2000. DOI: 10.1023/A:1008354220584.
- [7] Andreaus U., Casini P., Forced motion of friction oscillators limited by a rigid or deformable obstacle, *Mech. Struct. & Mach.*, 29(2), 177-198 (2001). DOI:10.1081/SME-100104479.
- [8] Andreaus U., Casini P., Forced response of a SDOF friction oscillator colliding with a hysteretic obstacle. Proceedings of DETC'01 ASME 2001 Design Engineering Technical Conferences and Computers and Information in Engineering Conference. Pittsburgh, Pennsylvania, September 9-12, 2001.
- [9] Andreaus U., Casini P., Friction oscillator excited by moving base and colliding with a rigid or deformable obstacle, *Int. J. of Nonlin. Mech.*, 37(1), pp. 117-133, 2002. DOI: 10.1016/S0020-7462(00)00101-3.
- [10] U. Andreaus, B. Chiaia, L. Placidi. Soft-impact dynamics of deformable bodies, *Continuum Mechanics and Thermodynamics*, Published online: August 2012, DOI: 10.1007/s00161-012-0266-5, Vol. 25(2-4), March 2013, pp. 375-398.
- [11] Ugo Andreaus, Maurizio De Angelis, Non-linear dynamic response of a base-excited SDOF oscillator with double-side unilateral constraints, *Nonlinear Dynamics* 2015 (accepted).
- [12] Andreaus U., Nistic N., An analytical-numerical model for contact-impact problems: theory and implementation in a two-dimensional distinct element algorithm, *Computer Modeling and Simulation in Engineering* 3(2), 1998, 98-110.
- [13] U. Andreaus, L. Placidi, G. Rega. Microcantilever dynamics in tapping mode atomic force microscopy via higher eigenmodes analysis. J.

of Applied Physics 113(22), 2013, Article number 224302, pages 1-14, Published 14 June 2013, DOI: 10.1063/1.4808446.

- [14] Andreaus U., Placidi L., Rega G. (2010). Numerical simulation of the soft contact dynamics of an impacting bilinear oscillator. *Communications in Nonlinear Science & Numerical Simulation*, Vol. 15(9); p. 2603-2616, doi: 10.1016/j.cnsns.2009.10.015.
- [15] U. Andreaus, L. Placidi and G. Rega. Soft impact dynamics of a cantilever beam: equivalent SDOF model versus infinite-dimensional system, , *Proc. of the Institution of Mechanical Engineers, Part C: J. of Mechanical Engineering Science* 2011 225(10): 2444-2456, DOI: 10.1177/0954406211414484.
- [16] A. Andreotti, D. Assante, F. Mottola, L. Verolino, Fast and accurate evaluation of the underground lightning electromagnetic field, *IEEE International Symposium on Electromagnetic Compatibility*, 2008.
- [17] A. Andreotti, D. Assante, A. Pierno, V.A. Rakov, R. Rizzo, A comparison between analytical solutions for lightning-induced voltage calculation, *Elektronika ir Elektrotechnika*, vol. 20(5), pp. 21-26, 2014.
- [18] A. Andreotti, D. Assante, V.A. Rakov, L. Verolino, Electromagnetic Coupling of Lightning to Power Lines: Transmission-Line Approximation versus Full-Wave Solution, *IEEE Trans. on EMC*, n 53 (2), pp. 421-428, 2011.
- [19] A. Andreotti, D. Assante, L. Verolino, Characteristic Impedance of Periodically Grounded Lossless Multiconductor Transmission Lines and Time-Domain Equivalent Representation, *IEEE Transactions on EMC*, 56 (1), pp. 221-230, 2014.
- [20] D. Assante, D. Davino, S. Falco, F. Schettino, L. Verolino, Coupling impedance of a charge traveling in a drift tube, *IEEE Transactions on Magnetics*, vol 41(5), pp. 1924-1927, 2005.
- [21] D. Assante, L. Verolino, Efficient evaluation of the longitudinal coupling impedance of a plane strip, *Progress In Electromagnetics Research M*, vol. 26, pp. 251-265, 2012.
- [22] R. Rizzo, A. Andreotti, D. Assante, A. Pierno, Characteristic impedance of power lines with ground wires — Impedancja charakterystyczna linii elektroenergetycznej z uziemieniem, *Przegląd Elektrotechniczny*, vol. 89(5), pp. 11-14, 2013.
- [23] D. Assante, A. Andreotti, L. Verolino, Considerations on the characteristic impedance of periodically grounded multiconductor transmission lines, *IEEE International Symposium on Electromagnetic Compatibility*, 2012.
- [24] Bilotta, A., Turco, E., A numerical study on the solution of the Cauchy problem in elasticity, *International Journal of Solids and Structures*, 46 (25-26), pp. 4451-4477, (2009)
- [25] Cazzani, A. (2013). On the dynamics of a beam partially supported by an elastic foundation: an exact solution-set. *International Journal of Structural Stability and Dynamics*, 13(08), 1350045.
- [26] Cesarano C., (2014). Generalized Chebyshev polynomials. *HACETTEPE JOURNAL OF MATHEMATICS AND STATISTICS*, vol. 43, p. 731-740, ISSN: 1303-5010
- [27] Cesarano C., (2012). Identities and generating functions on Chebyshev polynomials . *GEORGIAN MATHEMATICAL JOURNAL*, vol. 19, p. 427-440, ISSN: 1072-947X
- [28] Cesarano C., (2015). Integral representations and new generating functions of Chebyshev polynomials. *HACETTEPE JOURNAL OF MATHEMATICS AND STATISTICS*, vol. 44, p. 541-552, ISSN: 1303-5010, doi: 10.15672/HJMS.20154610029
- [29] Cesarano, C., Assante, D. A note on generalized Bessel functions, *International Journal of Mathematical Models and Methods in Applied Sciences*, 8 (1), pp. 38-42 (2014)
- [30] Cesarano C, Cennamo G..M, Placidi L., (2014). Humbert Polynomials and Functions in Terms of Hermite Polynomials Towards Applications to

Wave Propagation. WSEAS TRANSACTIONS ON MATHEMATICS, vol. 13, p. 595-602, ISSN: 1109-2769

- [31] Cesarano C, Cennamo G.M., Placidi L., (2014). Operational methods for Hermite polynomials with applications. WSEAS TRANSACTIONS ON MATHEMATICS, vol. 13, p. 925-931, ISSN: 1109-2769
- [32] Cesarano C., Fornaro C., (2015). OPERATIONAL IDENTITIES ON GENERALIZED TWO-VARIABLE CHEBYSHEV POLYNOMIALS. INTERNATIONAL JOURNAL OF PURE AND APPLIED MATHEMATICS, vol. 100, p. 59-74, ISSN: 1314-3395, doi: 10.12732/ijpam.v100i1.6
- [33] Cesarano C., Fornaro C., Vazquez L., (2015). A NOTE ON A SPECIAL CLASS OF HERMITE POLYNOMIALS. INTERNATIONAL JOURNAL OF PURE AND APPLIED MATHEMATICS, vol. 98, p. 261-273, ISSN: 1311-8080, doi: 10.12732/ijpam.v98i2.8
- [34] G. Dattoli, S. Lorenzutta, Cesarano C., (2001). Generalized Polynomials and New Families of Generating Functions. ANNALI DELL'UNIVERSIT DI FERRARA. SEZIONE 7: SCIENZE MATEMATICHE, vol. XLVII, p. 57-61, ISSN: 0430-3202
- [35] G. Dattoli, S. Lorenzutta, P.E. Ricci, Cesarano C., (2004). On a Family of Hybrid Polynomials. INTEGRAL TRANSFORMS AND SPECIAL FUNCTIONS, vol. 15, p. 485-490, ISSN: 1065-2469
- [36] Del Vescovo, D., Giorgio, I., Dynamic problems for metamaterials: Review of existing models and ideas for further research, International Journal of Engineering Science 80, pp. 153-172 (2014)
- [37] F. dell'Isola, H. Gouin, and G. Rotoli, Nucleation of spherical shell-like interfaces by second gradient theory: Numerical simulations, European Journal of Mechanics, B/Fluids, vol. 15 (4), 1996, pp. 545-568.
- [38] F. dell'Isola, H. Gouin, and P. Seppecher, Radius and surface tension of microscopic bubbles by second gradient theory, Comptes Rendus de l'Academie de Sciences - Serie IIb: Mecanique, Physique, Chimie, Astronomie, vol. 320 (6), 1995, pp. 211-216.
- [39] F. dell'Isola, M. Guarascio, and K. Hutter, A variational approach for the deformation of a saturated porous solid. A second-gradient theory extending Terzaghi's effective stress principle, Archive of Applied Mechanics, vol. 70 (5), 2000, pp. 323-337.
- [40] dell'Isola Francesco, Madeo Angela, Placidi L (2012). Linear plane wave propagation and normal transmission and reflection at discontinuity surfaces in second gradient 3D continua. ZEITSCHRIFT FUR ANGEWANDTE MATHEMATIK UND MECHANIK, vol. 92, p. 52-71, ISSN: 0044-2267, doi: 10.1002/zamm.201100022
- [41] F. dell'Isola, A. Madeo, and P. Seppecher, Boundary conditions at fluid-permeable interfaces in porous media: A variational approach, International Journal of Solids and Structures, vol. 46 (17), 2009.
- [42] F. dell'Isola and G. Rotoli, Validity of Laplace formula and dependence of surface tension on curvature in second gradient fluids, Mechanics Research Communications, vol. 22 (5), 1995, pp. 485-490.
- [43] F. dell'Isola, P. Seppecher and A. Della Corte "The postulations la D'Alembert and la Cauchy for higher gradient continuum theories are equivalent: a review of existing results", Proceedings of The Royal Society A, vol. 471 (2183), 2015,
- [44] F. dell'Isola, and D. Steigmann, A two-dimensional gradient-elasticity theory for woven fabrics, Journal of Elasticity, vol. 118 (1), 2015, pp. 113-125.
- [45] J.P. Den Hartog, Mechanical Vibrations, Dover Publications, New York, 1985.
- [46] Garusi, E., Tralli, A., Cazzani, A., An unsymmetric stress formulation for reissner-mindlin plates: A simple and locking-free rectangular

- element, *International Journal of Computational Engineering Science*, 5 (3), pp. 589-618 (2004)
- [47] Greco, L., Cuomo, M., Consistent tangent operator for an exact Kirchhoff rod model, *Continuum Mechanics and Thermodynamics* (in press)
- [48] J.B. Hunt, *Dynamic Vibration Absorbers*, Mechanical Engineering Publications, London, 1979.
- [49] D.J. Mead, *Passive Vibration Control*, Wiley, Chichester, 2000. D.J. Thompson, A continuous damped vibration absorber to reduce broad-band wave propagation in beams
- [50] A.D. Nashif, D.I.G. Jones, J.P. Henderson, *Vibration Damping*, Wiley, New York, 1985.
- [51] Ormondroyd, J. and Den Hartog, J.P., Theory of the dynamic vibration absorber, *Transactions of the American Society of Mechanical Engineers*, Vol. 50, pp. 9–22, 1928.
- [52] Piccardo, G., Ranzi, G., Luongo, A., A complete dynamic approach to the Generalized Beam Theory cross-section analysis including extension and shear modes, *Mathematics and Mechanics of Solids*, 19 (8), pp. 900-924 (2014)
- [53] Pignataro, M., Rizzi, N., Ruta, G., & Varano, V. (2010). The effects of warping constraints on the buckling of thin-walled structures. *Journal of Mechanics of Materials and Structures*, 4(10), 1711-1727.
- [54] Rizzi, N. L., Varano, V., & Gabriele, S. (2013). Initial postbuckling behavior of thin-walled frames under mode interaction. *Thin-Walled Structures*, 68, 124-134.
- [55] Placidi L, Giuseppe Rosi, Ivan Giorgio and Angela Madeo (2014). Reflection and transmission of plane waves at surfaces carrying material properties and embedded in second gradient materials. *MATHEMATICS AND MECHANICS OF SOLIDS*, vol. 19, p. 555-578, ISSN: 1081-2865, doi: 10.1177/1081286512474016
- [56] Rosi, G., Giorgio, I., Eremeyev, V.A., Propagation of linear compression waves through plane interfacial layers and mass adsorption in second gradient fluids, *ZAMM Zeitschrift für Angewandte Mathematik und Mechanik*, 93 (12), pp. 914-927 (2013)
- [57] Roveri, N., Carcaterra, A., Akay, A., Vibration absorption using non-dissipative complex attachments with impacts and parametric stiffness, *Journal of the Acoustical Society of America*, 126 (5), pp. 2306-2314, (2009)
- [58] Sadek, F., Mohraz, B., Taylor, A. W., & Chung, R. M. (1997). A method of estimating the parameters of tuned mass dampers for seismic applications. *Earthquake Engineering and Structural Dynamics*, 26(6), 617-636.
- [59] P. Seppecher, J.-J. Alibert and F. dell’Isola “Linear elastic trusses leading to continua with exotic mechanical interactions”, *Journal of Physics: Conference Series* vol. 319 (1), 2011, 13 pages.
- [60] Turco, E. (2001). An effective algorithm for reconstructing boundary conditions in elastic solids. *Computer methods in applied mechanics and engineering*, 190(29), 3819-3829.
- [61] Yang, Y., Misra, A., Micromechanics based second gradient continuum theory for shear band modeling in cohesive granular materials following damage elasticity, *International Journal of Solids and Structures*, 49 (18), pp. 2500-2514 (2012)